# LOAD AND RESISTANCE FACTOR DESIGN

DESIGN DATA

CLASS AA CONCRETE	f'c = 4,000	PSI
P.C. BEAM TYPE IV (80')	f'ci = 5,250	PSI
REINFORCING STEEL	Fy = 60,000	PSI
STRUCTURAL STEEL M270 (GRADE 50W) STAINLESS STEEL A240 (AUSTENITIC STAINLESS STEEL TYPE 316)	Fy = 50,000 Fy = 30,000	PSI

LOADING: HL-93 OR OKLAHOMA OVERLOAD TRUCK AND 20 LB. PER SQ. FT. FUTURE WEARING SURFACE AND 5 P.S.F. STAY-IN-PLACE FORMS.

DESIGN: AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 4th EDITION WITH 2009 INTERIMS ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE ANSI/AWS D1.6 STRUCTURAL WELDING CODE - STAINLESS STEEL

LFD OPERATING RATING: HS 42.7

# FOUNDATION DATA

## ABUTMENTS (HP 12x53 PILING)

### ABUTMENT NO. 1 & NO. 2 FACTORED PILE REACTION 1

91.0 TONS/PILE

### PIERS (5'-0" DIAMETER DRILLED SHAFTS)

	PIER NO. 1	PIER NO. 2	PIER NO. 3	PIER NO. 4
FACTORED REACTION	= 662.0 TONS/SHAFT	= 653.0 TONS/SHAFT	= 653.0 TONS/SHAFT	= 662.0 TONS/SHAFT
NOMINAL UNIT BEARING RESISTANCE BEARING RESISTANCE FACTOR FACTORED BEARING RESISTANCE	= 30.5 TONS/SQ.FT. = 0.7 = 419.2 TONS/SHAFT	= 42.4 TONS/SQ.FT. = 0.7 = 583.2 TONS/SHAFT	= 45.4 TONS/SQ.FT. = 0.7 = 624.0 TONS/SHAFT	= 25.0 TONS/SQ.FT. = 0.7 = 344.0 TONS/SHAFT
NOMINAL UNIT FRICTION RESISTANCE FRICTION RESISTANCE FACTOR FACTORED FRICTION RESISTANCE DEPTH OF ROCK NEGLECTED FOR FRICTION	- 6.3 TONS/SQ.FT. - 0.45 - 443.2 TONS/SHAFT - 5.0 FT.	<ul> <li>7.7 TONS/SQ.FT.</li> <li>0.45</li> <li>271.4 TONS/SHAFT</li> <li>5.0 FT.</li> </ul>	<ul> <li>7.1 TONS/SQ. FT.</li> <li>0.45</li> <li>252.3 TONS/SHAFT</li> <li>5.0 FT.</li> </ul>	= 9.0 TONS/SQ.FT. = 0.45 = 636.2 TONS/SHAFT = 5.0 FT.
TOTAL FACTORED RESISTANCE	= 862.4 TONS/SHAFT	= 854.6 TONS/SHAFT	= 876.3 TONS/SHAFT	= 980.2 TONS/SHAFT

 $^1$  all abutment piling shall be driven through the compacted fill. Piling shall be driven to point bearing on solid foundation material at the approximate elevation shown on the plans. If the required factored pile capacity is not obtained at this elevation, driving shall continue until the required factored pile capacity is obtained. The length of steel piling shown on the plans is for estimating purposes only.

	SUMM	IARY OF	QUANTITI	ES		
ITEM	UNIT	ABUTS.	PIERS	SUPERSTR.	APPR.	TOTAL
SUBSTRUCTURE EXCAVATION COMMON	C.Y.	180.0				180.0
CLSM BACKFILL	C.Y.	206.0				206.0
PRESTRESSED CONCRETE BEAMS (TYPE IV)	L.F.			1,595.0		1,595.0
APPROACH SLAB	S.Y.				281.2	281.2
SAW-CUT GROOVING	S.Y.			1,785.4	266.6	2,052.0
SEALED EXPANSION JOINT	L.F.			43.2		43.2
CONCRETE RAIL (TR4)	L.F.			803.4	120.0	923.4
STRUCTURAL STEEL	LB.			2,250.0		2,250.0
STAINLESS STEEL FIXED BEARING ASSSEMBLY	EA.			8.0		8.0
STAINLESS STEEL EXPANSION BEARING ASSEMBLY	EA.			32.0		32.0
CLASS AA CONCRETE	C.Y.			471.3		471.3
CLASS A CONCRETE	C.Y.	96.0	169.0			265.0
REINFORCING STEEL	LB.		1,350.0			1,350.0
EPOXY COATED REINFORCING STEEL	LB.	12,220.0	28,670.0	101,800.0		142,690.0
1) CLASS B BRIDGE DECK REPAIR	S.Y.					45.0
1) CLASS C BRIDGE DECK REPAIR	S.Y.					15.0
PILES, FURNISHED (HP 10X42)	L.F.	246.0				246.0
PILES, FURNISHED (HP 12X53)	L.F.	813.0				813.0
PILES, DRIVEN (HP 10X42)	L.F.	246.0				246.0
PILES, DRIVEN (HP 12X53)	L.F.	813.0				813.0
PILE SPLICE, H-PILE (NON-BIDDABLE)	EA.	1.0				1.0
WATER REPELLENT (VISUALLY INSPECTED)	S.Y.	96.0	479.0	1,412.0	56.0	2,043.0
DRILLED SHAFTS 60" DIAMETER	L.F.		430.0			430.0
CROSSHOLE SONIC LOGGING	EA.		2.0			2.0
SEALER CRACK PREPARATION	L.F.			122.4		122.4
SEALER RESIN	GAL.			1.5		1.5
(SP) NEST PREVENTION - NETTING	L.SUM					1.0
TYPE 1-A PLAIN RIPRAP	TON					3,220.0
TYPE 1-A FILTER BLANKET	TON					705.0
6" PERFORATED PIPE UNDERDRAIN ROUND	L.F.	84.0				84.0
6" NON-PERF. PIPE UNDERDRAIN RND.	L.F.	60.0				60.0
REMOVAL OF EXISTING BRIDGE STRUCTURE	L.SUM					1.0

(1) TO BE USED ON THE EXISTING BRIDGE AS DIRECTED BY THE ENGINEER.

	HYDRAULI	<u>C DATA</u>
	DRAINAGE AREA CONTROLLED AREA EFFECTIVE AREA	- 617.00 SQ. MI - 118.00 SQ. MI - 499.00 SQ. MI
Q2 V2 Q2 HIGHWATER	5,392.5 C.F.S. 2.65 F.P.S. EL. 945.31	Q50 30,786.72 C.F.S. V50 6.80 F.P.S. Q50 HIGHWATER EL. 954.34
Q5 V5 HIGHWATER Q10 V10 Q10 HIGHWATER	10,822.59 C.F.S. 3.95 F.P.S. EL. 948.05 15,366.43 C.F.S. 4.71 F.P.S. EL. 949.89	Q100 38,922.51 C.F.S. V100 7.25 F.P.S. Q100 HIGHWATER EL. 956.11 PIER SCOUR DEPTH = 8.42 FT. CONTRACTION SCOUR DEPTH = 67.92 FT. TOTAL SCOUR DEPTH = 76.34 FT.
Q25 V25 Q25 HIGHWATER	23,346.13 C.F.S. 5.86 F.P.S. EL. 952.42	Q-OT = Q50 = 30,795.55 C.F.S. PIER SCOUR DEPTH = 8.45 FT. CONTRACTION SCOUR DEPTH = 57.12 FT. TOTAL SCOUR DEPTH = 65.57 FT.





REV. NO. DESCRIPTION

DATE

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ты	DESIGN			U.S. 277 OVER DEEP RED CREEK COTTON COUNTY
••••	DRAWN			DESIGN DATA &
(. 73159	CHECKED			SUMMARY OF OUMNITITIES
591-3214	APPROVED			SUMMANT OF QUANTITIES
0/2010	SQUAD	G/K E	NGR.	JOB PIECE NO